

LEGEND

	BRICK		DOOR NUMBER SYMBOL
	CONCRETE MASONRY UNITS		WINDOW NUMBER SYMBOL
	GYPSUM BOARD PARTITIONS		ROOM NUMBER SYMBOL
	WOOD-FINISH GRADE		BUILDING SECTION SYMBOL
	WOOD BLOCKING		WALL SECTION/ELEVATION SYMBOL
	RIGID WALL/PERIMETER INSULATION		TITLE AND DETAIL REFERENCE SYMBOL
	RIGID ROOF INSULATION		WALL/BUILDING SECTION SYMBOL
	BATT INSULATION		TITLE AND DETAIL REFERENCE SYMBOL
	CONCRETE		PARTITION TYPES
	POROUS FILL		NUMBER-CONSTRUCTION NOTE
	EARTH		NUMBER-DEMOLITION NOTE
	METAL PATTERN		REVISION
			NORTH ARROW (CONSTRUCTION NORTH)

DRAWING INDEX

SHEET NAME	SHEET NUMBER	DRAWING TITLE
T1-01	1	TITLE SHEET
G1-01	2	INDEX, LEGEND AND ABBREVIATIONS
EC-01	3	EXISTING CONDITIONS (FOR REFERENCE ONLY)
A1-01	4	BASEMENT PLANS – DEMO AND NEW WORK
A1-02	5	FIRST AND SECOND FLOOR PLANS – NEW WORK
A5-01	6	SITE STAIR, BASEMENT DOOR, AND SIGNAGE
A5-02	7	LOUVER ELEVATION AND SECTION
S0-01	8	STRUCTURAL GENERAL NOTES AND CRITERIA
S1-01	9	BASEMENT PLAN AND FIRST FLOOR PLAN
S3-01	10	BUILDING SECTIONS
S3-02	11	BUILDING SECTIONS

ABBREVIATIONS

ABV	ABOVE	F	FILLER	N	NORTH
AD	ACCESS DOOR (OR PANEL)	FC	FAN COIL UNIT	NA	NOT APPLICABLE
ADA	AMERICAN WITH DISABILITIES ACT	FD	FLOOR DRAIN OR FIRE DAMPER	NIC	NOT IN CONTRACT
ADD	ADDENDUM	FE	FIRE EXTINGUISHER ON BRACKET	NO	NUMBER
ADJ	ADJACENT	FEC	FIRE EXTINGUISHER CABINET	NOM	NOMINAL
AES	ABOVE EXISTING SLAB	FH	FLAT HEAD	NTS	NOT TO SCALE
AF	ACCESS FLOOR	FIRE T	FIRE TREATED		
AFF	ABOVE FINISHED FLOOR	FIN	FINISH OR FINISHED	OA	OVERALL
AHU	AIR HANDLING UNIT	FIX	FIXTURE	OC	ON CENTER
ALT	ALTERNATE	FL	FLASHING	OHD	OVERHEAD COILING DOOR
ALUM	ALUMINUM	FLR	FLOOR	OHG	OVERHEAD COILING GRILLE
APPROX	APPROXIMATE	FR	FIRE RATED	OPNG	OPENING
ARCH	ARCHITECTURAL	FRC	FIBER-REINFORCED COATING	OPP	OPPOSITE
ATC	ACOUSTICAL TILE CEILING (CONCEALED SUSPENSION)	FT	FOOT OR FEET	OZ	OUNCE
AWP	ACOUSTICAL WALL PANEL	FTG	FOOTING		
				PAV	PAVER TILE
				PC	PIECE
BD	BOARD	GA	GAUGE	PF	PLASTIC FABRICATION
BEN	BENCH	GALV	GALVANIZED	PL	PLATE
BETW	BETWEEN	GEN	GENERAL	PLAM	PLASTIC LAMINATE
BLDG	BUILDING	GRD	GROUND	PLAS	PLASTER
BLKG	BLOCKING	GRT	GROUT	PREFAB	PREFABRICATED
BM	BEAM	GYP	GYPSPUM VENEER PLASTER	PRES T	PRESSURE TREATED
BOT	BOTTOM	GYPB	GYPSPUM BOARD (WALL OR CEILING)	PT	PAINT
BR	BRICK	GYPBS	GYPSPUM BOARD SHAFT-WALL ASSEMBLY	PTN	PARTITION
				PVC	POLYVINYL CHLORIDE
C/C	CENTER TO CENTER	H	HEAD		
CAB	CABINET	HB	HORIZONTAL BLIND	QTY	QUANTITY
CEM	CEMENT	HDW	HARDWARE		
CER	CERAMIC	HM	HOLLOW METAL	R	RISER OR RADIUS
CI	CAST IRON	HOR	HORIZONTAL	RCP	REINFORCED CONCRETE PIPE
CG	CORNER GUARD	HP	HIGH POINT	RD	ROOF DRAIN OR ROUND
CJ	CONTROL JOINT	HR	HOOR	REQ'D	REQUIRED
CL	CENTERLINE	HT	HEIGHT	REBAR	REINFORCING BAR
CLOS	CLOSET	HTR	HEATER	REINF	REINFORCED OR REINFORCING
CLG	CEILING	HVAC	HEATING, VENTILATING AND AIR CONDITIONING	RESF	RESINOUS FLOORING
CLR	CLEAR	HW	HOT WATER	REQ	REQUIRED
CMU	CONCRETE MASONRY UNIT			RET	RETURN
CO	CLEAR OPENING			REV	REVISION
COL	COLUMN	IN	INCH	RH	ROBE HOOK
COMP	COMPACTED	INSUL	INSULATION	RM	ROOM
CONC	CONCRETE	INT	INTERIOR	RO	ROUGH OPENING
CONSTR	CONSTRUCTION			RWR	RECESSED WASTE RECEPTACLE
CONT	CONTINUOUS	JT	JOINT	RV	ROOF VENT
CONV	CONVECTOR	L	LINTEL	RX	REMOVE EXISTING
CR	COLD ROLLED	LG	LONG	S	SILL, SOUTH OR SINGLE
CX	CONNECT TO EXISTING	LIN	LINOLEUM FLOOR COVERING	SCH	SCHEDULE OR SCHEDULED
		LLV	LONG LEG VERTICAL	SD	SOAP DISPENSER OR STORM DRAIN
D	DOUBLE	LOC	LOCATION	SECT	SECTION
DEG	DEGREE	LP	LOW POINT	SF	SQUARE FOOT
DEMO	DEMOLITION	LT	LIGHT	SFT	STRUCTURAL FACING TILE
DET	DETAIL	LTG	LIGHTING	SHT	SHEET
DIA	DIAMETER	LV	LOUVER	SIM	SIMILAR
DIR	DIRECTORY			SJ	STEEL JOIST
DN	DOWN			SND	SANITARY NAPKIN DISPOSAL
DO	DOOR OPENING	MACH	MACHINE	SOD	SECTIONAL OVERHEAD DOOR (STEEL;
DR	DOOR	MAS	MASONRY		ALUMINUM; PLASTIC PANEL)
DS	DOWNSPOUT	MATL	MATERIAL	SPEC	SPECIFICATION
DWG	DRAWING	MAX	MAXIMUM	SP	STAND PIPE
		MET	METAL	SSM	SOLID SURFACING MATERIAL
E	EAST	MDF	MEDIUM DENSITY FIBERBOARD	STAT	STATIONARY
EA	EACH	MFB	MINERAL FIBER BLANKET	STL	STEEL
EFS	EXTERIOR FINISH SYSTEM	MECH	MECHANICAL	STRUCT	STRUCTURAL OR STRUCTURE
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM	MET	METAL	SYS	SYSTEM
EJ	EXPANSION JOINT	MFR	MANUFACTURER		
EL	ELEVATION	MH	MANHOLE	T	TILE
ELEC	ELECTRIC OR ELECTRICAL	MIN	MINIMUM	T&B	TOP & BOTTOM
EPS	EXPANDED POLYSTYRENE	MISC	MISCELLANEOUS	T&G	TONGUE & GROOVE
EPX	EPOXY	MK	MARK		
EQ	EQUAL	MO	MASONRY OPENING		
EQUIP	EQUIPMENT	MP	METAL PANEL		
EST	ESTIMATE	MTD	MOUNTED		
EUH	ELECTRIC UNIT HEATER	MTL	METAL		
EW	EACH WAY				
EW	ELECTRIC WATER COOLER				
EWCA	ELECTRIC WATER COOLER - ACCESSIBLE				
EXIST	EXISTING				
EXP	EXPANSION OR EXPOSED				
EXT	EXTERIOR				

PROFESSIONAL CERTIFICATION.
I HEREBY CERTIFY THAT THESE DOCUMENTS
WERE PREPARED OR APPROVED BY ME, AND
THAT I AM A DULY LICENSED PROFESSIONAL
ENGINEER UNDER THE LAWS OF THE STATE
OF MARYLAND. LICENSE NO. _____
EXPIRATION DATE: XX/XX/XXXX



Whitman, Requardt & Associates, LLP
801 South Caroline Street, Baltimore, Maryland 21231

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GENERAL NOTES

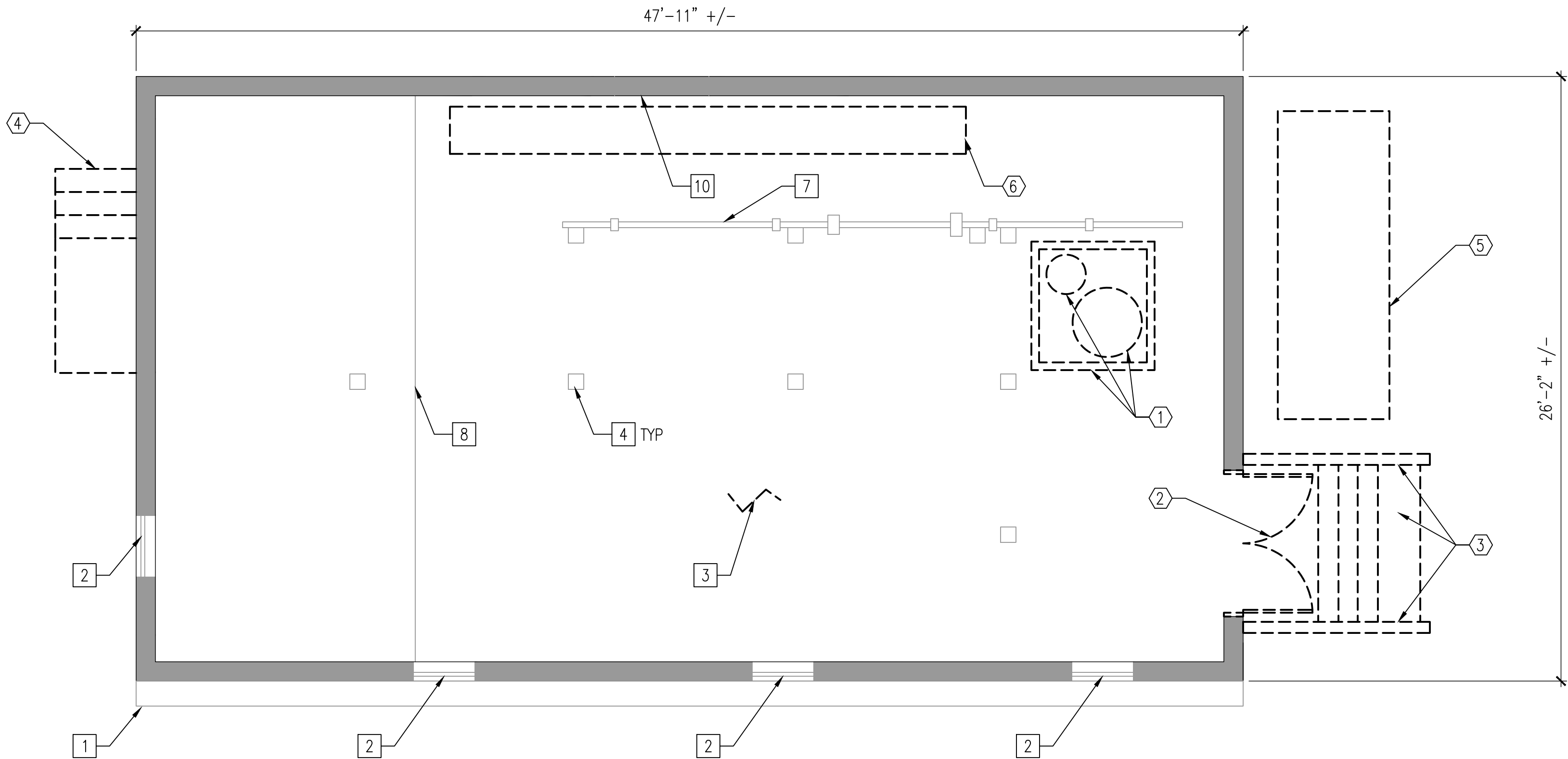
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DEMOLITION KEYNOTES

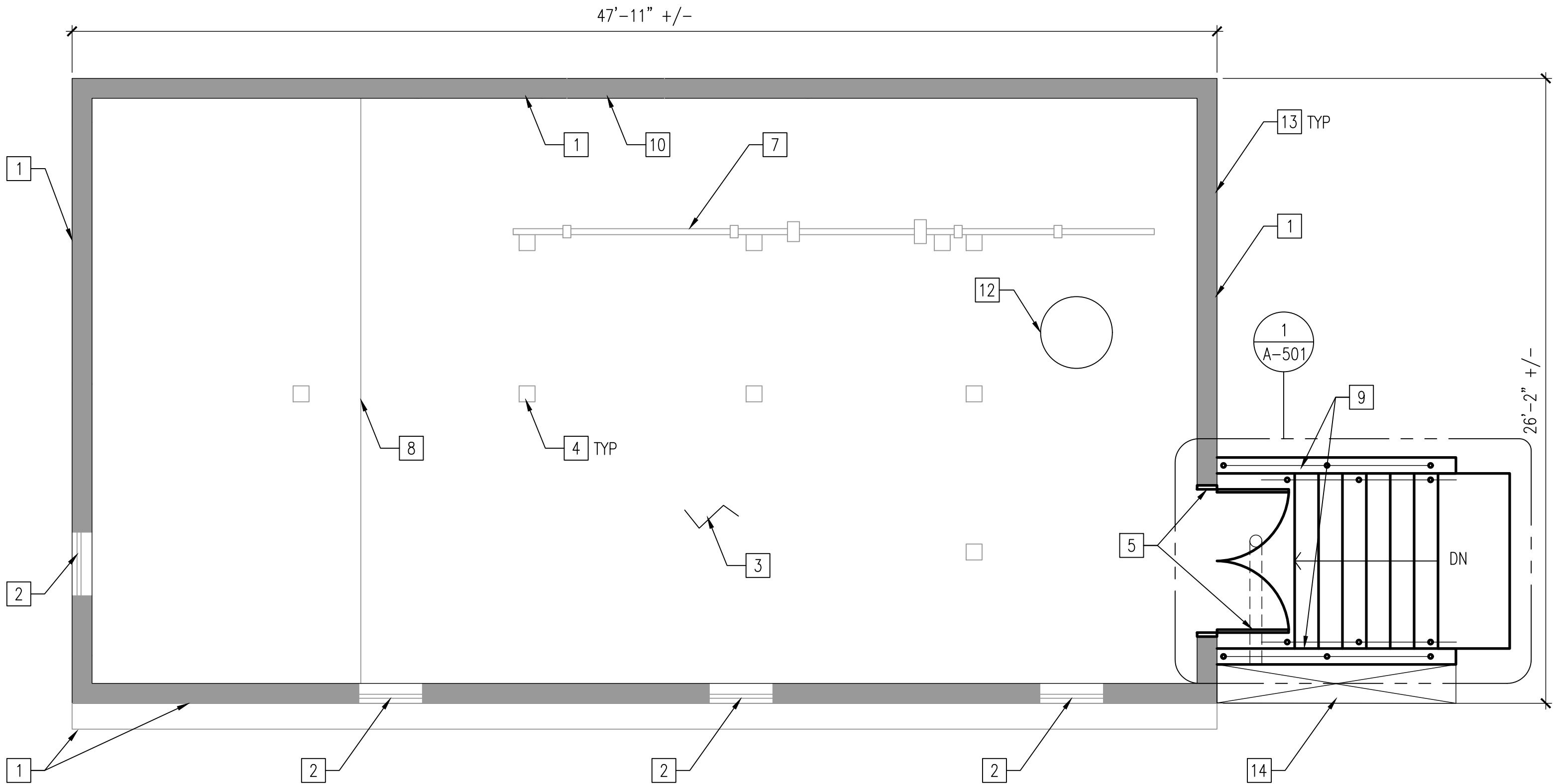
- ① DEMOLISH EXISTING WELL EQUIPMENT AND WOOD WALLS AROUND WELL EQUIPMENT. CAP EXISTING WELL.
- ② DEMOLISH EXISTING DOORS AND FRAME
- ③ DEMOLISH EXISTING CONCRETE STAIRS AND CHEEK WALLS. EXCAVATE AROUND EXISTING STAIRS TO ALLOW CONSTRUCTION OF NEW STAIRS (SEE STRUCTURAL DRAWINGS). TEMPORARILY BENCH, SLOPE, OR SHORE EXCAVATION AS NEEDED (SUPPORT OF EXCAVATION TO BE DESIGNED AND PROVIDED BY THE CONTRACTOR).
- ④ REMOVE AND DISPOSE OF EXISTING STEEL STAIR PLATFORM.
- ⑤ REMOVE AND DISPOSE OF EXISTING METAL SHED AND SLAB.
- ⑥ DEMOLISH, REMOVE, AND DISPOSE OF EXISTING LARGE CONCRETE MASS (SEE STRUCTURAL DRAWINGS S1-01, S3-01, AND S3-02).

NEW WORK KEYNOTES

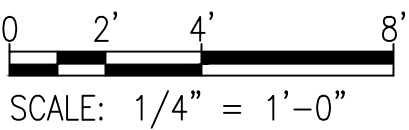
- 1 EXISTING CONCRETE WALL
- 2 EXISTING WOOD WINDOW FRAME
- 3 EXISTING UNFINISHED, UNGRADED DIRT FLOOR
- 4 EXISTING WOOD POST
- 5 STEEL DOOR AND FRAME
- 6 CONCRETE STAIRS AND CHEEK WALLS
- 7 HISTORIC MILL EQUIPMENT INCLUDING PULLEY WHEELS, CRANKSHAFT, AND ALL HARDWARE TO REMAIN IN PLACE
- 8 EXISTING LEDGE OF SOIL IN CRAWLSPACE
- 9 CONCRETE CHEEK WALLS, STEPS, AND 1 1/2" DIA. GALVANIZED METAL HANDRAILS. TOP OF CHEEK WALLS 6" ABOVE AND PARALLEL TO GRADE. SEE STRUCTURAL DRAWINGS FOR DETAILS
- 10 FIRST FLOOR DOOR OPENING ABOVE
- 11 2'-0" X 2'-0" LOUVER, GALVANIZED FINISH, TYP
- 12 CAPPED WELL
- 13 PROVIDE POSITIVE-SIDE WATERPROOFING SYSTEM FROM W.R. MEADOWS TO CONCRETE FOUNDATION (PROVIDE WATERPROOFING PRIMER, FLUID-APPLIED WATERPROOFING MEMBRANE, DRAINAGE SHEET, AND PROTECTION COURSE)
- 14 LOCATION OF FILTER CLOTH WRAPPED DRYWELL FILLED WITH #57 GRAVEL



1 HOYLE'S MILL BASEMENT PLAN - DEMOLITION
A1-01 SCALE: 1/4"=1'-0"



2 HOYLE'S MILL BASEMENT PLAN - NEW WORK
A1-01 SCALE: 1/4"=1'-0"



\$\$\$\$\$DISPERSE\$\$\$\$\$
\$DATE\$B\$

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801 South Caroline Street, Baltimore, Maryland 21231

xx/xx/xxx

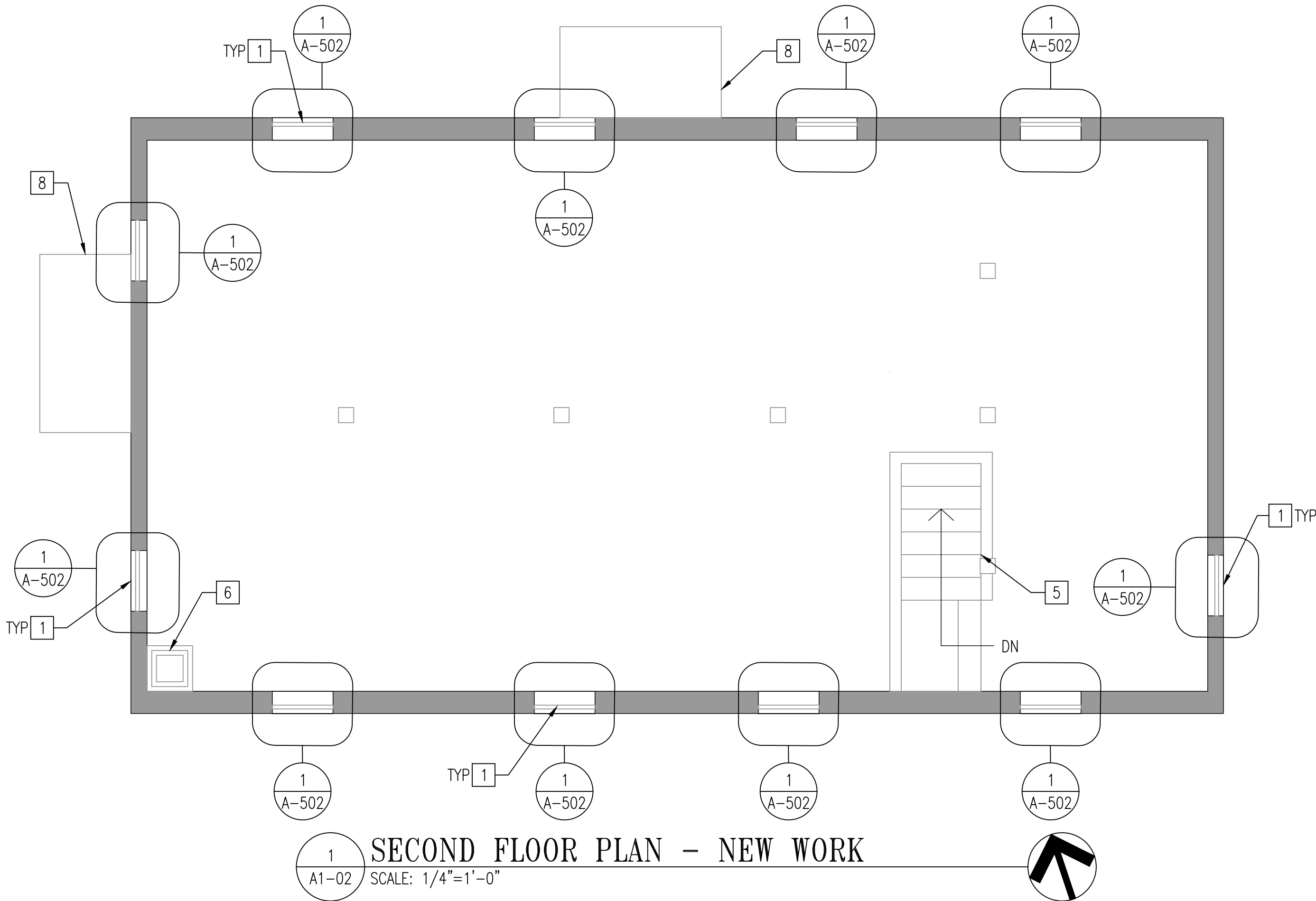
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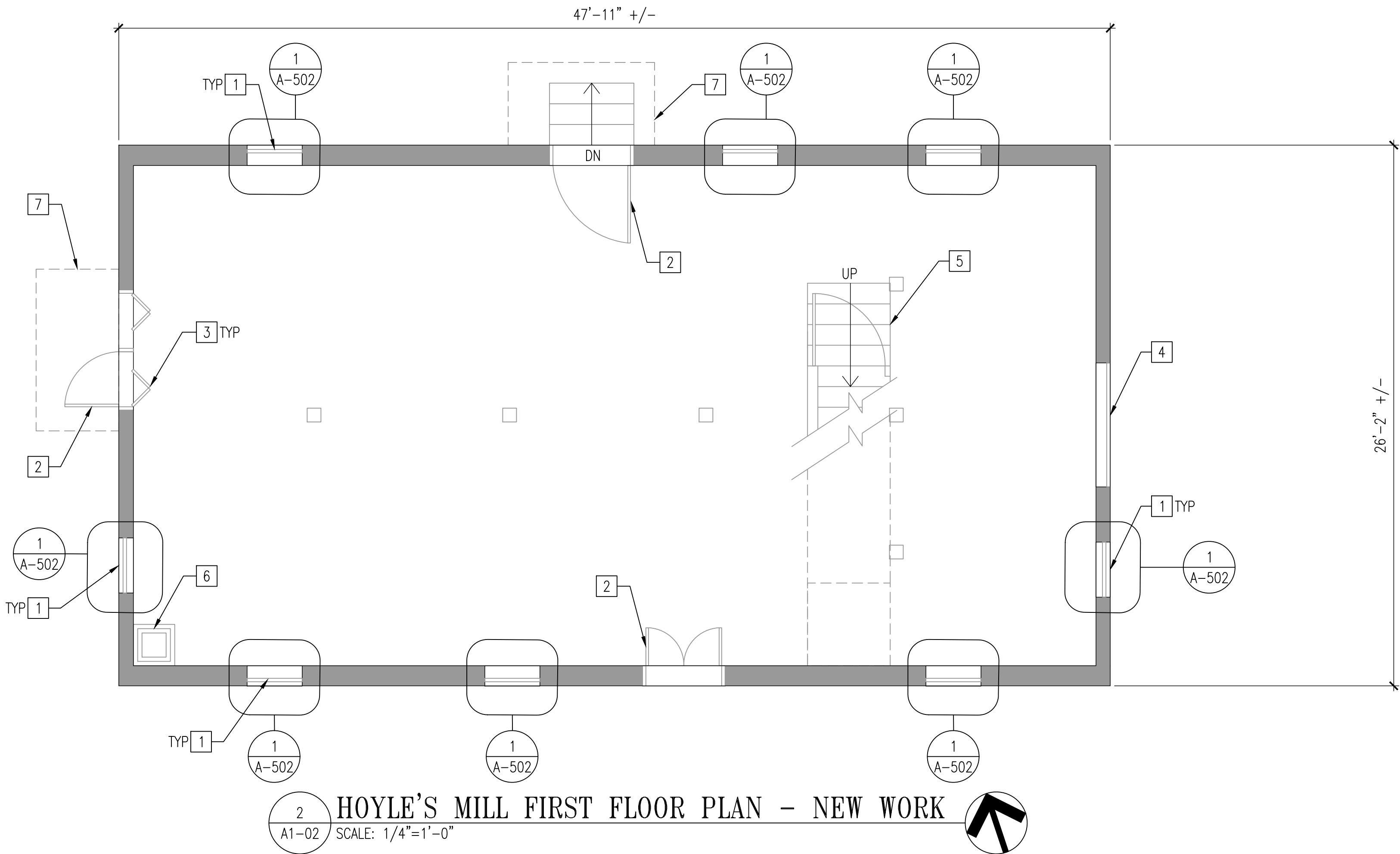
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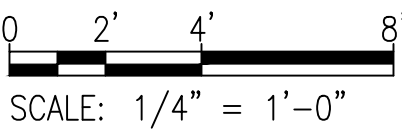
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- 2 EXISTING WOOD DOOR
- 3 EXISTING INTERIOR BI-FOLD DOOR
- 4 FORMER DOOR OPENING
- 5 STAIR WELL
- 6 EXISTING BRICK CHIMNEY
- 7 CANOPY ABOVE
- 8 CANOPY



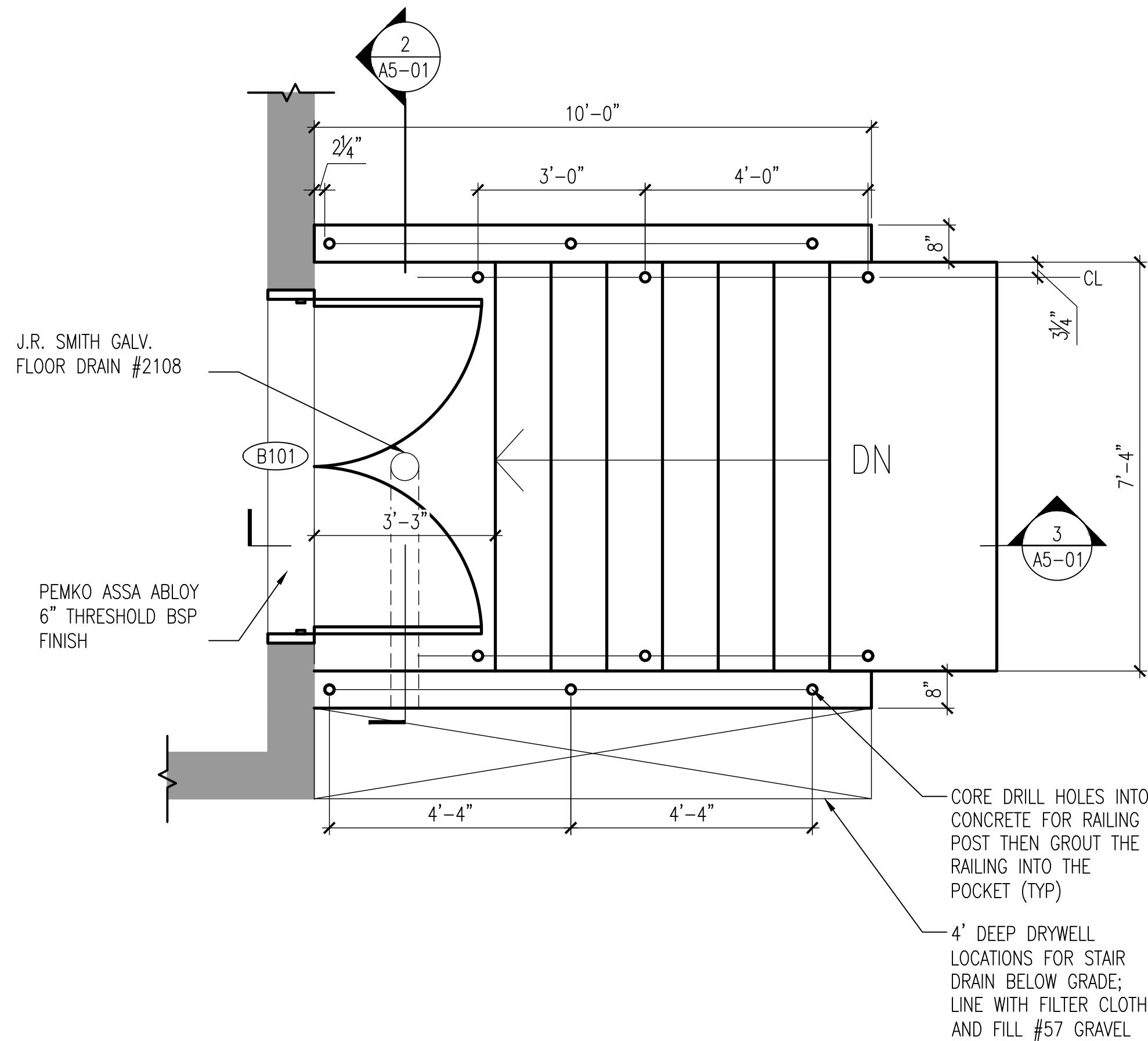
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A1-02 SCALE: 1/4"=1'-0"

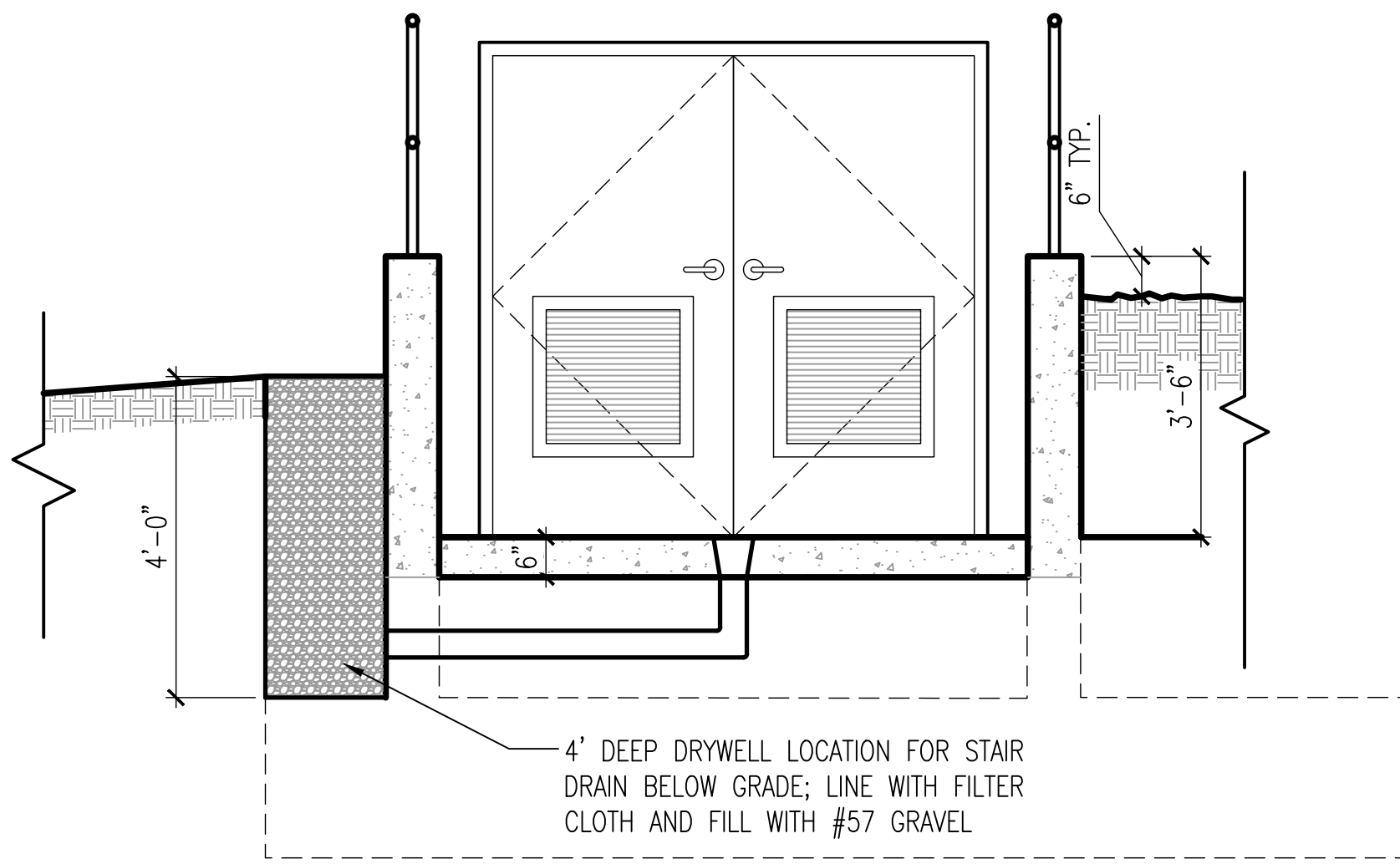


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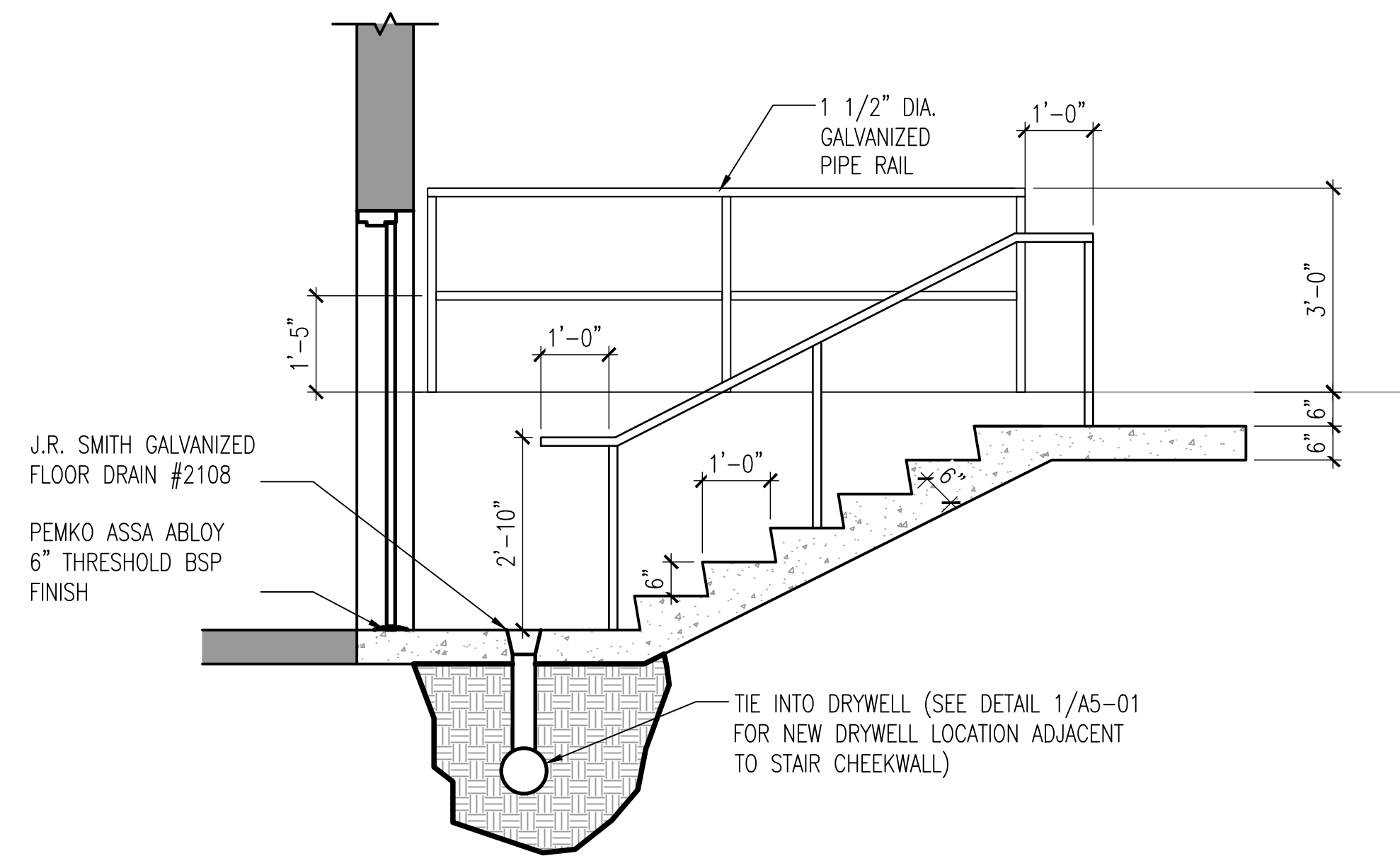


1 ENLARGED STAIR PLAN
A5-01 SCALE: 1/2"=1'-0"

DOOR NO.	DOORS						FRAMES		
	SIZE (NOMINAL)	LOCATION	TYPE	MATERIAL	FINISH	RATING	TYPE	SIZE	FINISH
B101	6'-0" X 6'-0" V.I.F.	BASEMENT	1	STEEL	GALVANIZED STEEL	N/A	HM	64" X 62"	GALVANIZED STEEL

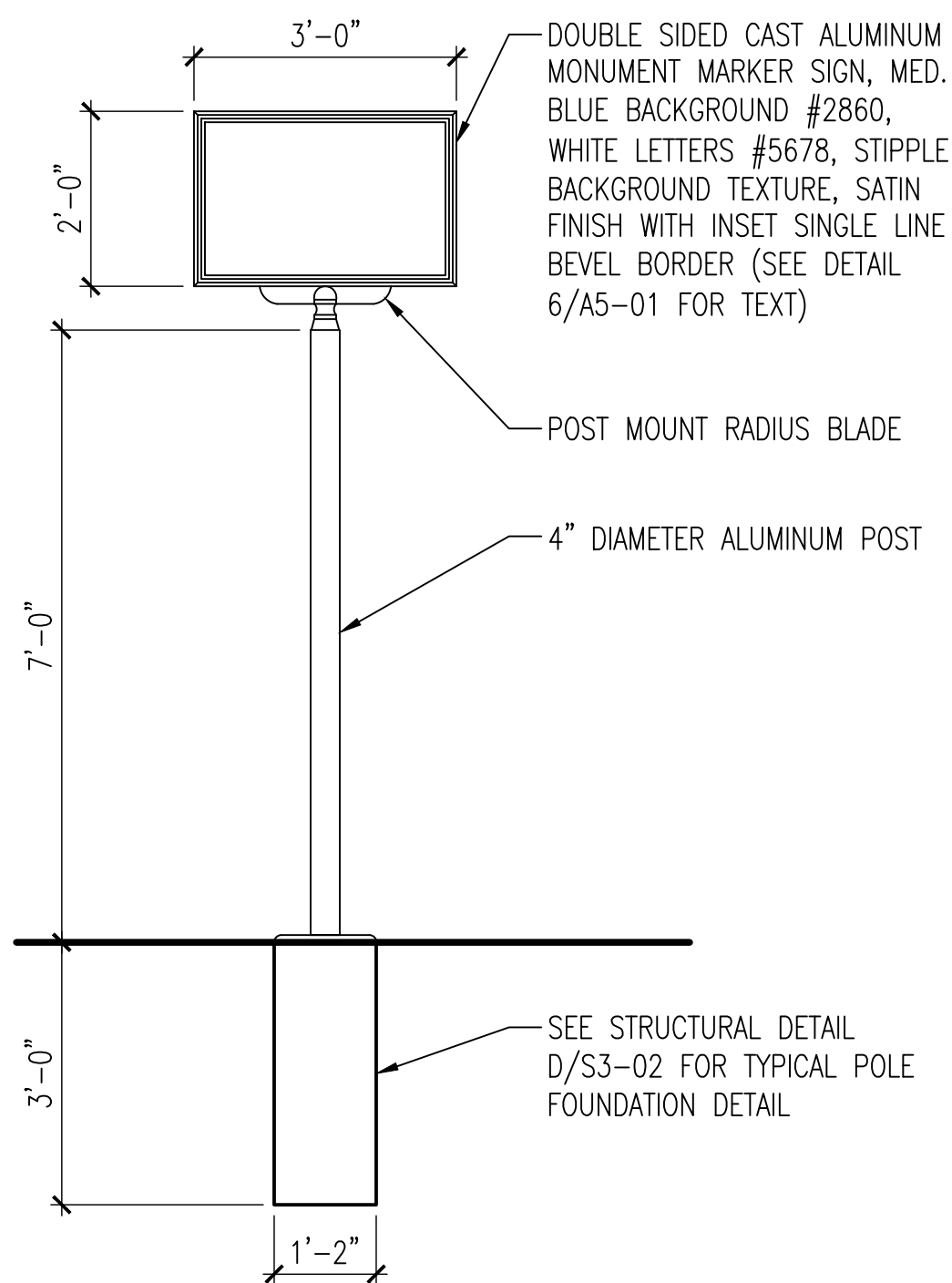


2 DOOR ELEVATION AND CHEEK WALL CROSS SECTION
A5-01 SCALE: 1/2"=1'-0"

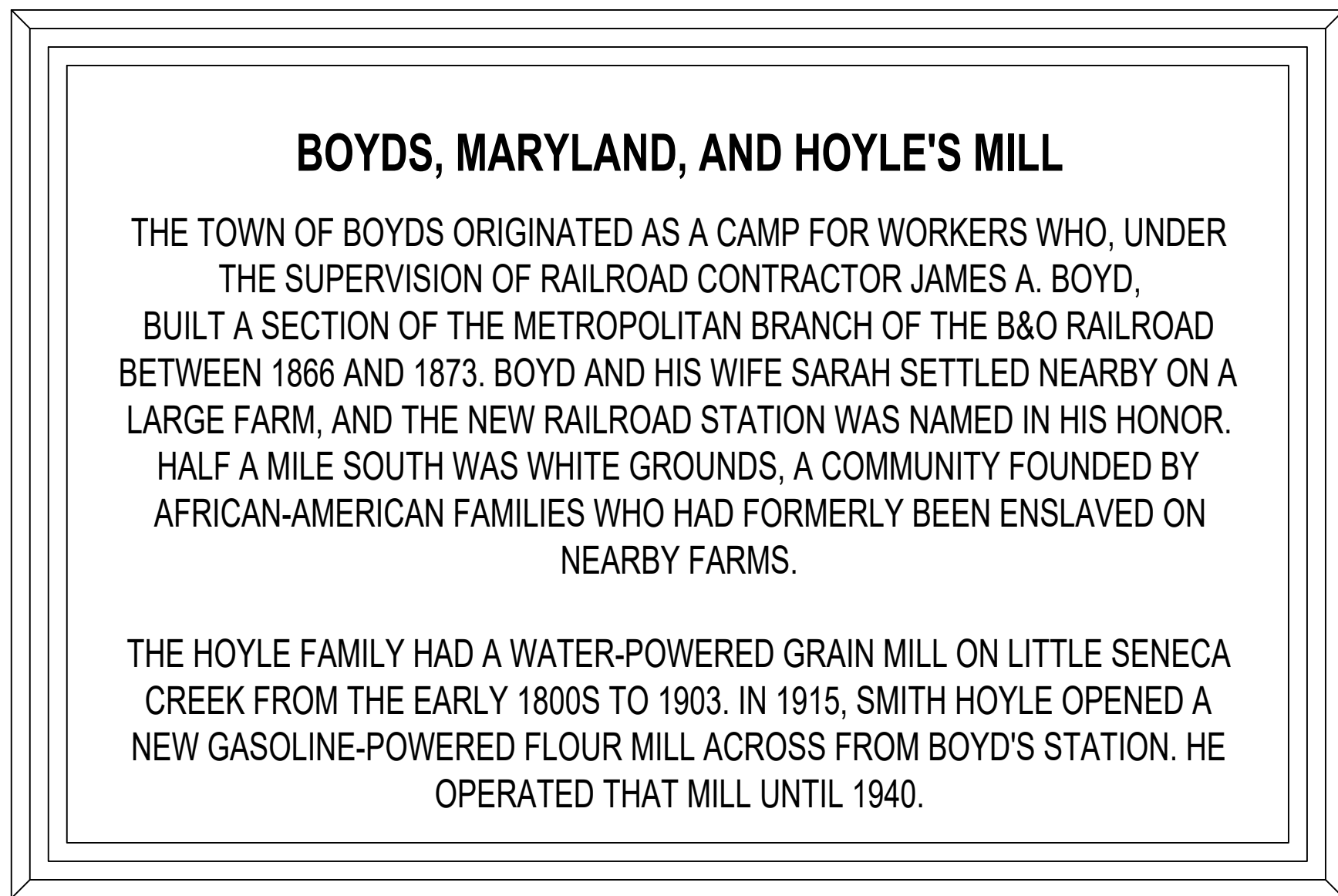


NOTES: GUARDRAILS AND HANDRAILS ARE GALVANIZED STEEL WITH 2" ROUND DIAMETER. DESIGN OF GUARDRAILS AND HANDRAILS IS DELEGATED TO THE CONTRACTOR.

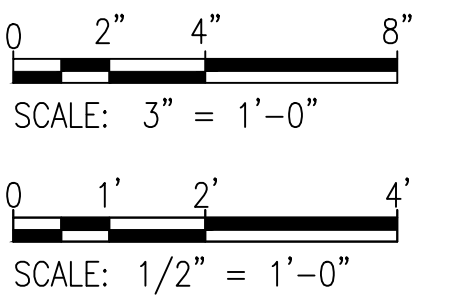
3 DOOR AND STAIR CROSS SECTION
A5-01 SCALE: 1/2"=1'-0"



4 HISTORICAL MARKER SIGN ELEVATION
A5-01 SCALE: 1/2"=1'-0"



6 HISTORICAL MARKER SIGN ELEVATION - ENLARGED
A5-01 SCALE: 3"=1'-0"



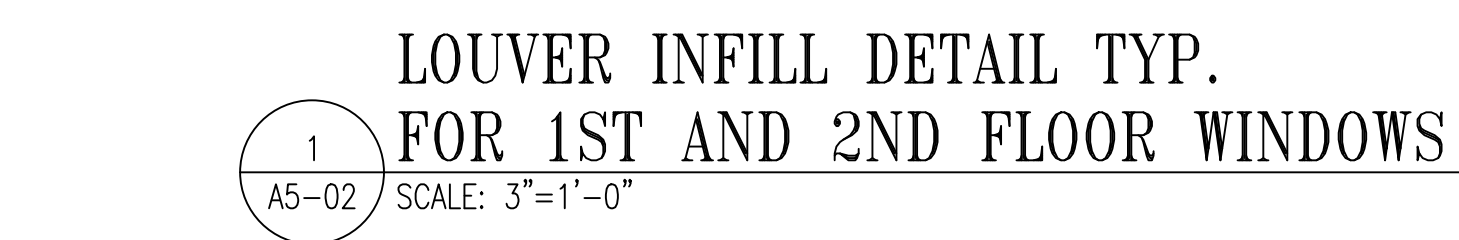
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Whitman, Requardt & Associates, LLP
801 South Caroline Street, Baltimore, Maryland 21231

xx/xx/xxxx

MONTGOMERY COUNTY DEPARTMENT OF TRANSPORTATION GAITHERSBURG, MARYLAND				A5-01 SITE STAIR, BASEMENT DOOR, AND LOUVERS HOYLE'S MILL STRUCTURAL STABILIZATION BOYDS, MARYLAND	
RECOMMENDED FOR APPROVAL					
Chief, Transportation Planning and Design Section				Date	
APPROVED				Date	
Chief, Division of Transportation Engineering				Date	
Designed by: <u>LH</u> Drawn by: <u>FILE</u> Checked by: <u>FAH</u>				SCALE : AS NOTED	
NO. REVISION DATE BY				Project No. : <u>32207.003</u>	
				SHEET <u>6</u> of 11	

30 JAN 2025



GENERAL STRUCTURAL NOTES

GENERAL

1. THE SCOPE OF THIS PROJECT IS TO REINFORCE AND STABILIZE THE EXISTING HOYLE'S MILL STRUCTURE.
2. STABILIZATION OF THE HOYLE'S MILL STRUCTURE MUST OCCUR PRIOR TO THE MODIFICATIONS ASSOCIATED WITH THE TRANSPORTATION PROJECT AT THIS SITE. (TRANSPORTATION WORK IS NOT IN CONTRACT.)
3. STRUCTURAL STABILIZATION WORK IN THESE DOCUMENTS WILL NOT MAKE HOYLE'S MILL SUITABLE FOR OCCUPANCY. IT IS ASSUMED BUILDING WILL REMAIN UNOCCUPIED. ADDITIONAL STRUCTURAL MODIFICATIONS THAT ARE OUTSIDE OF THE SCOPE OF THIS PROJECT WOULD BE REQUIRED FOR A FUTURE ADAPTIVE REUSE OF THIS BUILDING.
4. FIELD VERIFY DIMENSIONS, LOCATIONS AND ELEVATIONS SHOWN ON DRAWINGS FOR EXISTING STRUCTURES. BRING DISCREPANCIES TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
5. AVOID DAMAGING EXISTING HOYLE'S MILL BUILDING DURING THE WORK. ANY DAMAGES TO HOYLE'S MILL CAUSED BY THE CONTRACTOR MUST BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
6. COORDINATE ACTIVITIES WITH THE OWNER.
7. THE DRAWINGS SHOW THE FINAL CONDITION OF THE STRUCTURE. PROVIDE MEANS TO STABILIZE THE STRUCTURE DURING TEMPORARY CONDITIONS.
8. SCALES NOTED ON THE DRAWINGS ARE FOR GENERAL INFORMATION ONLY. DO NOT OBTAIN DIMENSIONAL INFORMATION FROM DIRECT SCALING OF THE DRAWINGS.
9. SPECIAL INSPECTIONS MUST BE PERFORMED BY A THIRD-PARTY SPECIAL INSPECTOR HIRED BY THE OWNER. SPECIAL INSPECTIONS MUST BE IN ACCORDANCE WITH THE MONTGOMERY COUNTY DEPARTMENT OF PERMITTING SERVICES SPECIAL INSPECTIONS PROGRAM AND THE STATEMENT OF SPECIAL INSPECTIONS FOR THIS PROJECT.
10. DUE TO THE CONDITION OF THE FOUNDATION OF HOYLE'S MILL, THE GRADING DIRECTLY IN FRONT OF THE BUILDING ON THE NORTH AND EAST ELEVATIONS IS UNSTABLE. UNTIL THE NEW BASEMENT WALLS ARE CONSTRUCTED AND BACKFILLED AGAINST, CONTRACTOR IS RECOMMENDED TO AVOID PARKING AND DRIVING VEHICLES NEAR THE BUILDING TO PREVENT SOIL FROM MIGRATING INTO THE HOYLE'S MILL CRAWLSPACE. THE APPROPRIATE SAFE DISTANCE FROM THE BUILDING FACE MUST BE DETERMINED BY THE CONTRACTOR AS PART OF THEIR MEANS AND METHODS.
11. VIBRATION CAUSED BY CONSTRUCTION OF THE HELICAL PILES OR BY SUPPORT VEHICLES MUST NOT EXCEED ACCEPTABLE VIBRATION LEVELS CONTAINED IN THE PRECONSTRUCTION SURVEY WRITTEN REPORT FOR HOYLE'S MILL BUILDING OR THE VIBRATION LEVELS PROVIDED UNDER SECTION 400-02.03.04, WHICHEVER ARE LOWER.

SHALLOW FOUNDATIONS

1. DESIGN OF SHALLOW FOUNDATIONS IS IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL MEMORANDUM BY WRA DATED FEBRUARY 1, 2024.
2. SHALLOW FOUNDATIONS MUST BEAR UPON UNDISTURBED SOIL OR COMPACTED ENGINEERED FILL WITH A MINIMUM ALLOWABLE BEARING CAPACITY OF 3000 PSF. OBTAIN THE SERVICES OF A GEOTECHNICAL ENGINEER LICENSED IN THE STATE OF MARYLAND WHO IS RESPONSIBLE FOR VERIFICATION OF THE SPECIFIED MINIMUM ALLOWABLE BEARING CAPACITY AT EACH FOOTING.
3. SHALLOW FOUNDATION ELEVATIONS SHOWN ON THE DRAWINGS ARE MINIMUM EXCAVATION DEPTHS. EXCAVATE FURTHER AS REQUIRED TO REMOVE UNSATISFACTORY SOILS TO A LAYER WITH THE MINIMUM SPECIFIED ALLOWABLE BEARING CAPACITY. WHERE REQUIRED, PROVIDE COMPACTED ENGINEERED FILL TO ACHIEVE THE REQUIRED SUBGRADE ELEVATIONS. NOTIFY THE ENGINEER OF ANY CONDITIONS THAT REQUIRE CHANGES IN FOUNDATION ELEVATIONS.
4. PLACE SHALLOW FOUNDATIONS ON THE SAME DAY THAT THE BEARING SURFACE IS INSPECTED BY THE CONTRACTOR'S GEOTECHNICAL ENGINEER. ANY BEARING SURFACE NOT PLACED ON THE SAME DAY OF INITIAL INSPECTION MUST BE RE-INSPECTED BY THE CONTRACTOR'S GEOTECHNICAL ENGINEER ON THE DAY CONCRETE IS PLACED.
5. KEEP EXCAVATIONS DRY.
6. MINIMUM DEPTH BELOW GRADE FOR BOTTOM OF FOUNDATIONS FOR FROST PROTECTION IS 30 INCHES.
7. DO NOT PLACE BACKFILL AGAINST SUBSTRUCTURE WALLS UNTIL WALL CONCRETE ACHIEVES ITS SPECIFIED 28-DAY COMPRESSIVE STRENGTH USING FIELD-CURED CYLINDERS.
8. PROVIDE SUPPORT OF EXCAVATIONS REQUIRED TO COMPLETE THE WORK SHOWN ON THE DRAWINGS. SUPPORT OF EXCAVATION SYSTEMS MUST BE DESIGNED BY THE CONTRACTOR'S PROFESSIONAL ENGINEER.

CONCRETE

1. PROVIDE NORMAL-WEIGHT CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS.
2. CONCRETE MUST BE AIR ENTRAINED.
3. DETAIL AND CONSTRUCT REINFORCED CONCRETE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE ACI 301, "SPECIFICATION FOR STRUCTURAL CONCRETE", AND AS SPECIFIED HEREIN.
4. DETAIL REINFORCING STEEL IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" AND ACI SP-66, "ACI DETAILING MANUAL."
5. PROVIDE REINFORCING STEEL CONFORMING TO ASTM A615, GRADE 60, DEFORMED BARS.
6. UNLESS NOTED OTHERWISE ON THE DRAWINGS, PROVIDE CONCRETE COVER FOR REINFORCING STEEL AS FOLLOWS:
 - A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH, FILL: 3"
 - B. CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH: EARTH OR FILL: 2"
7. SUBMIT REINFORCING STEEL DETAILS AND JOINT LAYOUT (SHOP DRAWINGS) AND RECEIVE APPROVAL FROM THE ENGINEER BEFORE PROCEEDING WITH FABRICATION.
8. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNLESS NOTED OTHERWISE.
9. COLD WEATHER PLACEMENT OF CONCRETE MUST BE IN ACCORDANCE WITH ACI 306R, ACI 306.1, AND THE SPECIFICATIONS.
10. HOT WEATHER PLACEMENT OF CONCRETE MUST BE IN ACCORDANCE WITH ACI 305R, ACI 305.1, AND THE SPECIFICATIONS.

ADHESIVE ANCHORS

1. THE ADHESIVE ANCHOR SYSTEM USED FOR POST INSTALLED ANCHORAGE TO CONCRETE MUST CONFORM TO THE REQUIREMENTS OF THE MOST RECENTLY PUBLISHED ACI 355.4, "ACCEPTANCE CRITERIA FOR QUALIFICATION OF POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE AND COMMENTARY." EACH ADHESIVE ANCHOR SYSTEM MUST SATISFY THE STRENGTH REQUIREMENTS FOR ITS USE. BULK-MIXED ADHESIVES ARE NOT PERMITTED. ADHESIVE ANCHORAGE DESIGN IS IN ACCORDANCE WITH ACI 318-14. ADHESIVE ANCHORS IN CONCRETE MUST BE QUALIFIED FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.4. PROVIDE THE FOLLOWING ANCHOR SYSTEMS, OR APPROVED EQUALS:
 - A. ANCHORAGE TO CONCRETE
 - i. HILTI HIT-HY 200 V3 WITH HILTI HIT-Z-R ROD OR HAS-R THREADED ROD.
 2. PROVIDE TYPE 316 STAINLESS STEEL ANCHORS.
 3. CONCRETE AT THE TIME OF ADHESIVE ANCHOR INSTALLATION MUST HAVE A MINIMUM AGE OF 21 DAYS.
 4. INSTALL ADHESIVE ANCHORS WITH A MINIMUM EDGE DISTANCE OF 3 INCHES TO ANY FREE EDGE OF CONCRETE, OR EDGE DISTANCE INDICATED ON DRAWINGS, WHICHEVER IS GREATER.
 5. INSTALL ADHESIVE ANCHORS WITH TRAINED QUALIFIED PERSONNEL, IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
 6. PROVIDE THOROUGHLY CLEANED ANCHOR HOLES PRIOR TO ADHESIVE INJECTION, AS REQUIRED BY THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. PROTECT DRILLED AND CLEANED ANCHOR HOLES FROM CONTAMINATION UNTIL THE ADHESIVE IS INSTALLED.
 7. PROVIDE ANCHORS CLEAN, OIL-FREE, AND FREE OF LOOSE RUST, PAINT, OR OTHER COATINGS.
 8. PROVIDE INSTALLED ADHESIVE ANCHORS SECURELY FIXED IN-PLACE TO PREVENT DISPLACEMENT WHILE THE ADHESIVE CURES.
 9. THE ENGINEER MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
 10. DO NOT DAMAGE EXISTING REINFORCING STEEL IN THE CONCRETE DURING ANCHOR INSTALLATION, UNLESS OTHERWISE NOTED ON THE DRAWINGS. PRIOR TO ANCHOR INSTALLATION, DETERMINE LOCATION OF EXISTING REINFORCING STEEL BY NON-DESTRUCTIVE MEANS AND NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN REINFORCING STEEL AND ANCHOR LOCATION PRIOR TO FABRICATION OF MATERIALS.
 11. DESIGN BASIS FOR ADHESIVE ANCHORS IS INDICATED ABOVE. SUBSTITUTIONS WILL BE CONSIDERED, BUT PRODUCT MUST MEET OR EXCEED ALL CRITERIA OF THE SPECIFIED ANCHOR. SUBSTITUTION REQUESTS MUST BE APPROVED IN WRITING BY THE ENGINEER PRIOR TO USE. PROVIDE PRODUCT DATA AND CALCULATIONS DEMONSTRATING THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURES AND INSTALLATION TEMPERATURE.

HELICAL PILES

1. HELICAL PILE FOUNDATIONS MUST BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MARYLAND, WITH AT LEAST 10 YEARS EXPERIENCE IN THE DESIGN OF HELICAL PILES. SUBMIT QUALIFICATIONS FOR REVIEW AND APPROVAL.
2. HELICAL PILE FOUNDATIONS MUST BE 2-1/4 INCH DIAMETER SS225 PILE SERIES HELICAL PILE SHAFTS OR APPROVED EQUAL. PROVIDE MAXIMUM HELIX PLATE DIAMETER OF 14".
3. PROVIDE HELICAL PILES AT THE LOCATIONS AND ELEVATIONS SHOWN WITH A MINIMUM ULTIMATE COMPRESSIVE CAPACITY OF 53 KIPS PER PILE. HELICAL PILES MUST HAVE AN ALLOWABLE COMPRESSIVE CAPACITY OF 17.5 KIPS PER PILE. ALLOWABLE PILE COMPRESSIVE CAPACITIES MUST BE ESTABLISHED AND VERIFIED BASED ON FINAL INSTALLATION TORQUE WITH A MINIMUM FACTOR OF SAFETY OF 3.0.
4. HELICAL PILE FOUNDATIONS MUST EXTEND A MINIMUM LENGTH OF 23.0 FEET BELOW THE BOTTOM OF THE EXISTING CONCRETE WALL TO BE SUPPORTED.
5. INSTALLATION OF HELICAL PILE FOUNDATIONS MUST BE PERFORMED BY A QUALIFIED EXPERIENCED INSTALLER WHO SPECIALIZES IN HELICAL PILE WORK. INSTALLER MUST HAVE A MINIMUM OF FIVE YEARS OF EXPERIENCE INSTALLING HELICAL PILES ON PROJECTS OF SIMILAR SIZE AND SCOPE. PROVIDE A LIST OF AT LEAST THREE PROJECTS USING HELICAL PILE FOUNDATIONS COMPLETED WITHIN THE LAST FIVE YEARS.
6. INSTALLATION EQUIPMENT MUST BE A ROTARY TYPE, HYDRAULIC POWER-DRIVEN TORQUE MOTOR WITH CLOCKWISE AND COUNTER-CLOCKWISE ROTATION CAPABILITY. MOTOR MUST BE A HIGH TORQUE, LOW REVOLUTIONS PER MINUTE (RPM) MOTOR WITH THE ABILITY TO ADJUST RPM DURING INSTALLATION. TORQUE MOTOR MUST HAVE A TORQUE CAPACITY AT LEAST 15 PERCENT GREATER THAN THE FINAL INSTALLATION TORQUE REQUIRED FOR THE PROJECT.
7. CONTRACTOR MUST PREPARE AND SUBMIT TO THE ENGINEER FOR APPROVAL, HELICAL PILE MANUFACTURER AND PRODUCT INFORMATION, INSTALLER QUALIFICATIONS AND EQUIPMENT, AND HELICAL PILE FOUNDATION DESIGN ENGINEER QUALIFICATIONS AT LEAST 30 CALENDAR DAYS PRIOR TO THE PLANNED START OF INSTALLATION.
8. CONTRACTOR'S APPROVED HELICAL PILE FOUNDATION DESIGN ENGINEER MUST PREPARE AND SUBMIT TO THE ENGINEER FOR APPROVAL, A HELICAL PILE FOUNDATION DESIGN REPORT WITH CALCULATIONS, SHOP DRAWINGS, DETAILS AND SPECIFICATIONS FOR THE HELICAL PILES PROPOSED FOR USE ON THIS PROJECT; PLANNED PILE INSTALLATION DEPTH AND CONFIGURATION, MINIMUM FINAL PILE INSTALLATION TORQUE, AND REQUIREMENTS FOR CORROSION PROTECTION AT LEAST 14 CALENDAR DAYS PRIOR TO THE PLANNED START OF INSTALLATION.
9. THE AVERAGE TORQUE FOR THE LAST THREE FEET OF PENETRATION MUST BE USED AS THE BASIS OF COMPARISON WITH THE MINIMUM INSTALLATION TORQUE AS REQUIRED. THE AVERAGE TORQUE MUST BE DEFINED AS THE AVERAGE OF THE LAST THREE READINGS RECORDED AT ONE FOOT INTERVALS.
10. UPON COMPLETION OF THE HELICAL PILE INSTALLATION, PROVIDE THE OWNER WITH COPIES OF THE PILE INSTALLATION LOGS, MAINTAINED DURING CONSTRUCTION, CONFIRMING THE HELICAL PILE CONFIGURATION, DEPTH OF INSTALLATION, AND RECORDED FINAL INSTALLATION TORQUE VALUES.
11. PROVIDE THE OWNER WITH A CERTIFICATION PREPARED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MARYLAND, STATING THAT THE HELICAL PILES AS INSTALLED HAVE THE CAPACITY TO SUPPORT THE STRUCTURE AND THAT THE ALLOWABLE HELICAL PILE CAPACITIES HAVE BEEN VERIFIED.

RECOMMENDED SEQUENCES OF DEMOLITION AND CONSTRUCTION

1. INSTALL HELICAL PILES TO SUPPORT EXISTING NORTH AND EAST WALLS.
2. INSTALL TEMPORARY SHORING OF FIRST FLOOR FRAMING WITHIN CRAWLSPACE.
3. INSTALL STEEL CHANNELS AND ANGLES WITH THROUGH-BOLTS ALONG EXISTING CONCRETE WALLS ON NORTH AND EAST ELEVATIONS.
4. REMOVE ALL TRASH AND DEBRIS, AND WELL ENCLOSURE AND EQUIPMENT, FROM CRAWLSPACE AREA. CAP AND ABANDON EXISTING WELL.
5. DEMOLISH EXISTING STONE MASONRY FOOTINGS FOR NORTH AND EAST WALLS, DEMOLISH EXISTING LARGE CONCRETE MASS, DEMOLISH METAL SHED AND CONCRETE PAD UNDER METAL SHED, DEMOLISH EXISTING SITE STAIR AND CHEEK WALLS, AND EXCAVATE BELOW WALLS TO THE BOTTOM ELEVATION OF THE NEW FOOTINGS.
6. INSTALL REINFORCEMENT, FORMWORK, AND PLACE CONCRETE FOR CONTINUOUS WALL FOOTINGS FOR NORTH AND EAST WALLS.

RECOMMENDED SEQUENCES OF DEMOLITION AND CONSTRUCTION (CONTINUED)

7. FOR THE SECTION OF THE SOUTH WALL FOUNDED ON A STONE MASONRY FOOTING, DEMOLISH THE STONE MASONRY FOOTING, EXCAVATE TO THE BOTTOM ELEVATION OF THE NEW FOOTINGS, AND PLACE NEW CONCRETE FOOTINGS. PERFORM THIS WORK IN STAGES TO LIMIT THE LENGTH OF THE EXISTING FOOTING REMOVED AT ANY ONE TIME, AND PROVIDE TEMPORARY SHORING AS NEEDED.
8. CONSTRUCT NEW CONCRETE SITE STAIR AND CHEEK WALLS.
9. PERFORM THE REMAINDER OF THE WORK INDICATED WITHIN THE CONTRACT DOCUMENTS.

DESIGN LOADS AND CRITERIA

ALL LOADS INDICATED BELOW ARE UNFACTORED. LOADS INDICATED BELOW WERE USED FOR THE DESIGN OF THE FOUNDATION ELEMENTS FOR THE STRUCTURAL STABILIZATION DETAILED IN THESE DRAWINGS. EXISTING SUPERSTRUCTURE HAS NOT BEEN ANALYZED FOR ITS CAPACITY TO SUPPORT THESE LOADS. DESIGN OF THE STRUCTURAL STABILIZATION WORK ASSUMES BUILDING WILL REMAIN UNOCCUPIED. RE-ANALYSIS OF THE STRUCTURE WILL BE REQUIRED FOR ANY FUTURE ADAPTIVE REUSE OF THE BUILDING.

1. RISK CATEGORY: II
2. DEAD LOADS:
 - A. STRUCTURES: ACTUAL WEIGHT
 - B. WEIGHT OF SOIL – 100 PCF FOR RESISTING UPLIFT
 - C. WEIGHT OF SOIL – 120 PCF FOR DEAD LOAD
 - D. EARTH PRESSURE COEFFICIENTS:
 - i. AT-REST: $K_0 = 0.5$
 - ii. ACTIVE: $K_A = 0.33$
 - iii. PASSIVE: $K_P = 3.0$
 - E. SUPERIMPOSED DEAD LOAD:
 - i. ROOF: 20 PSF
 - ii. 2ND FLOOR: 20 PSF
 - iii. 1ST FLOOR: 20 PSF
 - iv. SUPERIMPOSED DEAD LOAD INCLUDES COMBINED WEIGHT OF ALL ASSUMED PERMANENT NON-STRUCTURAL COMPONENTS SUPPORTED BY THE FRAMING, INCLUDING MEP COMPONENTS, ROOFING, FLOOR AND CEILING FINISHES, AND SPRINKLERS FOR A FUTURE ADAPTIVE REUSE OF THE BUILDING.
3. LIVE LOADS: (DESIGN LIVE LOADS USED FOR THE DESIGN OF THE FOUNDATION MODIFICATIONS. SUPERSTRUCTURE HAS NOT BEEN ANALYZED TO VERIFY LIVE LOAD CAPACITY)
 - A. FIRST FLOOR: 100 PSF
 - B. SECOND FLOOR: 100 PSF
 - C. ATTIC SPACE: 100 PSF
 - D. STAIRS AND LANDINGS: 100 PSF
 - E. GUARDRAILS AND HANDRAILS – 200 LBS AT EACH POST OR 50 PLF ALONG THE TOP RAIL, WHICHEVER IS GREATER.
4. ROOF LIVE LOAD: 30 PSF
5. ROOF SNOW LOAD:
 - A. GROUND SNOW LOAD (PG): 30 PSF
 - B. EXPOSURE FACTOR (CE): 1.0
 - C. THERMAL FACTOR (CT): 1.2
 - D. SNOW LOAD IMPORTANCE FACTOR (IS): 1.0
 - E. SLOPED ROOF SNOW LOAD (PS): 25.2 PSF
6. WIND LOAD:
 - A. ULTIMATE WIND SPEED (VULT): 115 MPH
 - B. NOMINAL WIND SPEED (VASD): 89 MPH
 - C. EXPOSURE CATEGORY: C
 - D. INTERNAL PRESSURE COEFFICIENT: +/- 0.18
 - E. COMPONENTS AND CLADDING: PER ASCE 7-16
7. SEISMIC LOAD:
 - A. SEISMIC IMPORTANCE FACTOR (IE): 1.0
 - B. MAXIMUM EARTHQUAKE SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS: $S_S=0.135g$
 - C. MAXIMUM EARTHQUAKE SPECTRAL RESPONSE ACCELERATION AT ONE-SECOND: $S_1=0.043g$
 - D. SITE CLASSIFICATION: D
 - E. SITE SEISMIC COEFFICIENT: $FA=1.6$; $FV=2.4$
 - F. SPECTRAL RESPONSE COEFFICIENTS: $SDS = 0.144$; $SD1 = 0.069$
 - G. SEISMIC DESIGN CATEGORY: B
 - H. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
 - I. BASIC SEISMIC FORCE RESISTING SYSTEM: LIGHT-FRAME (WOOD) BEARING WALLS WITH SHEAR PANELS.
 - J. RESPONSE MODIFICATION FACTOR: $R=2$

DELEGATED DESIGN

1. DESIGN AND DETAILING RESPONSIBILITY FOR THE FOLLOWING ENGINEERED SYSTEMS AND COMPONENTS IS DELEGATED TO A QUALIFIED PROFESSIONAL ENGINEER, SELECTED AND HIRED BY THE CONTRACTOR. THESE SYSTEMS AND COMPONENTS INCLUDE, BUT ARE NOT LIMITED TO:
 - A. GUARDRAILS AND HANDRAILS
 - B. TEMPORARY SUPPORT OF EXCAVATION AND STRUCTURES
 - C. CONCRETE FORMWORK AND SHORING
 - D. HELICAL PILES
 2. DELEGATED DESIGN ITEMS MUST COMPLY WITH THE APPLICABLE DESIGN CODES, STANDARDS, CRITERIA, AND LOADS INDICATED IN THE CONSTRUCTION DOCUMENTS.
 3. PROVIDE CALCULATIONS AND SHOP DRAWINGS FOR DELEGATED DESIGN ITEMS, STAMPED AND SIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MARYLAND, UNLESS OTHERWISE NOTED. SUBMIT CALCULATIONS AND SHOP DRAWINGS FOR REVIEW AND APPROVAL.
- CODES AND STANDARDS**
1. MARYLAND BUILDING PERFORMANCE STANDARDS (MBPS).
 2. INTERNATIONAL BUILDING CODE IBC (2018), INCLUDING THE MODIFICATIONS MADE BY LOCAL JURISDICTION
 3. INTERNATIONAL EXISTING BUILDING CODE IEBG (2018), INCLUDING THE MODIFICATIONS MADE BY LOCAL JURISDICTION
 4. AMERICAN CONCRETE INSTITUTE ACI 318 (2014), "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
 5. AMERICAN SOCIETY OF CIVIL ENGINEERS ASCE 7 (2016), "MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES"

STRUCTURAL ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ARCH	ARCHITECT
APPROX	APPROXIMATE
BOT	BOTTOM
CLR	CLEAR
COL	COLUMN
CONC	CONCRETE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
DEMO	DEMOLITION/DEMOLISH
DET	DETAIL
DWG	DRAWING
EQ	EQUAL
EL	ELEVATION
ENGR	ENGINEER
EXIST	EXISTING
EF	EACH FACE
EW	EACH WAY
FDN	FOUNDATION
FT	FOOT/FEET
HORIZ	HORIZONTAL
IBC	INTERNATIONAL BUILDING CODE
MIN	MINIMUM
PSI	POUNDS PER SQUARE INCH
ROW	RIGHT OF WAY
STRUCT	STRUCTURAL
TYP	TYPICAL
VIF	VERIFY IN FIELD
VERT	VERTICAL

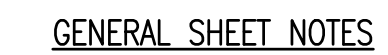
\$\$\$DGN SPEC\$\$\$
TO A YEAR\$

PROFESSIONAL CERTIFICATION.
I HEREBY CERTIFY THAT THESE DOCUMENTS
WERE PREPARED OR APPROVED BY ME, AND
THAT I AM A DULY LICENSED PROFESSIONAL
ENGINEER UNDER THE LAWS OF THE STATE
OF MARYLAND. LICENSE NO. _____
EXPIRATION DATE: XX/XX/XXXX



Whitman, Requardt & Associates, LLP
801 South Carolina Street, Baltimore, Maryland 21231

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1. REFER TO SHEET S0-01 FOR STRUCTURAL GENERAL NOTES, BUILDING CODES AND STANDARDS, AND DESIGN LOADS.
2. ALL DIMENSIONS INDICATED ARE APPROXIMATE. FIELD VERIFY DIMENSIONS, LOCATIONS AND ELEVATIONS SHOWN ON DRAWINGS FOR EXISTING STRUCTURES. BRING DISCREPANCIES TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH WORK.
3. COORDINATE WORK WITH ARCHITECTURAL DRAWINGS.
4. THE EXISTING STRUCTURE IS IN VERY POOR CONDITION, AND ACCESS TO THE BUILDING IS DANGEROUS. CONTRACTOR MUST PROVIDE HELICAL PILES AND TEMPORARY SHORING WITHIN THE BASEMENT AREA PRIOR TO BEGINNING WORK WITHIN THE BASEMENT. HELICAL PILES AND TEMPORARY SHORING MUST BE DESIGNED AND PROVIDED BY THE CONTRACTOR.
5. THERE IS NO STRUCTURAL SCOPE ON THE SECOND FLOOR NOR THE ROOF.

X SHEET KEYNOTES

1. EXISTING SITE RETAINING WALL ADJACENT TO EXTERIOR WALL TO REMAIN.
2. EXISTING EXTERIOR CONCRETE WALL ON MASONRY FOOTINGS TO REMAIN.
3. EXISTING WOOD BUILDING COLUMN TO REMAIN.
4. EXISTING WINDOW OPENING TO REMAIN.
5. DEMOLISH EXISTING STAIR AND CHEEK WALLS (SEE SHEET A1-01). PROVIDE NEW STAIR, CHEEK WALLS, AND GUARDRAILS AND HANDRAILS (SEE SHEET A5-01). DESIGN OF GUARDRAILS AND HANDRAILS IS DELEGATED TO CONTRACTOR'S ENGINEER.
6. ELEVATION CHANGE IN EXISTING UNFINISHED BASEMENT FLOOR.
7. 10'-0" LONG X 1'-6" WIDE X 4'-0" DEEP DRYWELL. WRAP PERIMETER OF DRYWELL WITH GEOTECHNICAL FILTER FABRIC AND FILL WITH #57 GRAVEL.
8. HELICAL PILES SPACED AT 6'-0" +/- TO SUPPORT EXISTING CONCRETE FOUNDATION WALL. DESIGN OF HELICAL PILE FOUNDATION SYSTEM IS DELEGATED TO CONTRACTOR'S ENGINEER.
9. DEMOLISH EXISTING DOUBLE DOOR. PROVIDE NEW DOUBLE DOOR. SEE SHEET A5-01.
10. EXISTING DOOR TO REMAIN.
11. EXISTING EXTERIOR WOOD-FRAMED WALLS TO REMAIN.
12. EXISTING FULL-DEPTH, FULL-WIDTH CRACK IN EXISTING CONCRETE WALL. PROVIDE CONTINUOUS SUPPORT PER DETAIL A/S3-02 ALONG NORTH AND EAST WALLS PRIOR TO DEMOLITION OF EXISTING STONE RUBBLE FOUNDATIONS FOR THESE WALLS.
13. DEMOLISH, REMOVE, AND DISPOSE OF EXISTING LARGE CONCRETE MASS.
14. PATCH HOLE IN FLOOR LOCATED IN FRONT OF EXISTING STAIR WITH 3/4" THICK PLYWOOD.
15. EXISTING 1/2" THICK WOOD BOARDS ON 2"x8" WOOD JOISTS SPACED AT 22" +/- ON CENTER.
16. EXISTING 7" WIDE X 9" DEEP WOOD GIRDER.
17. DIMENSION INDICATES EXTENTS OF EXISTING 6' +/- TALL CONCRETE WALL. IN THIS REGION, KEEP EXISTING CONCRETE WALL. DEMOLISH EXISTING STONE MASONRY FOUNDATION AND PROVIDE NEW CONCRETE FOUNDATION SIMILAR TO FOUNDATION IN SECTION B/S3-01.
18. DEMOLISH EXISTING WELL EQUIPMENT AND WELL ENCLOSURE. CAP EXISTING WELL AND ABANDON IN PLACE.
19. CONTINUOUS FOUNDATION DRAIN (FRENCH DRAIN), SLOPE TO DRYWELL IN SOUTHEAST CORNER OF BUILDING FOOTPRINT.
20. 6" THICK CONCRETE STAIR LANDING WITH #4@12 EACH WAY, TOP BARS. SLOPE STAIR LANDING TO DRAIN, TIE DRAIN PIPE TO CONTINUOUS FOUNDATION DRAIN.
21. TERMINATE END OF THE FOUNDATION DRAIN INTO THE DRYWELL.
22. 6" THICK CONCRETE STAIR LANDING WITH #4@12 EACH WAY.



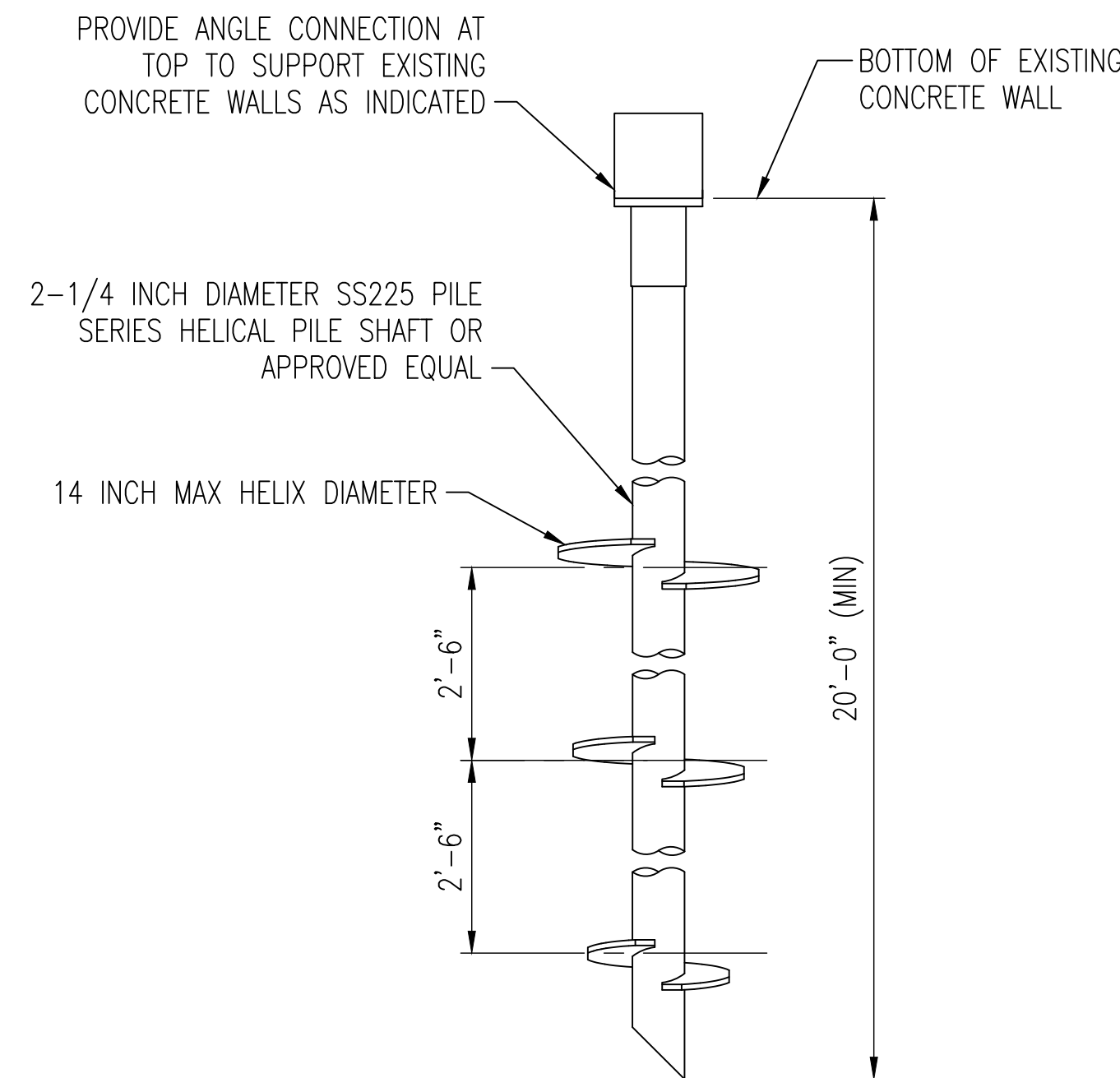
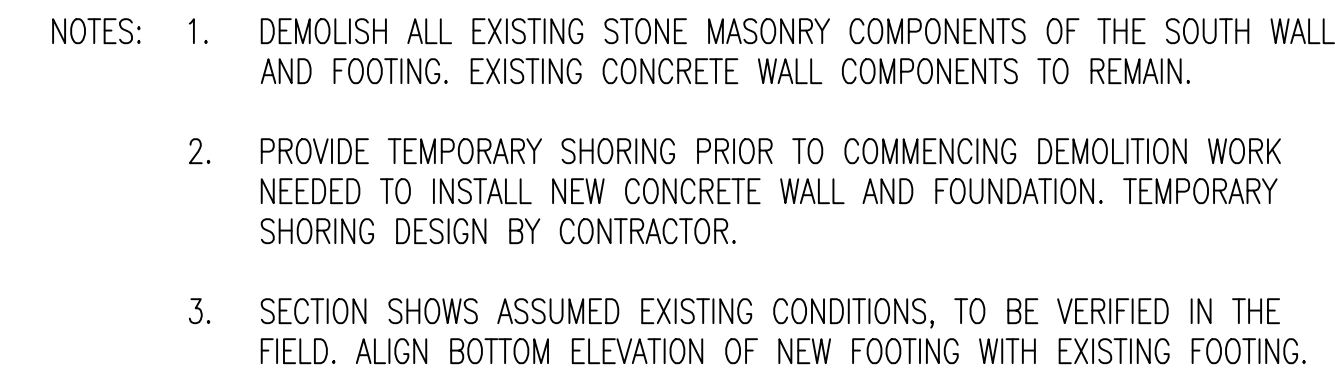
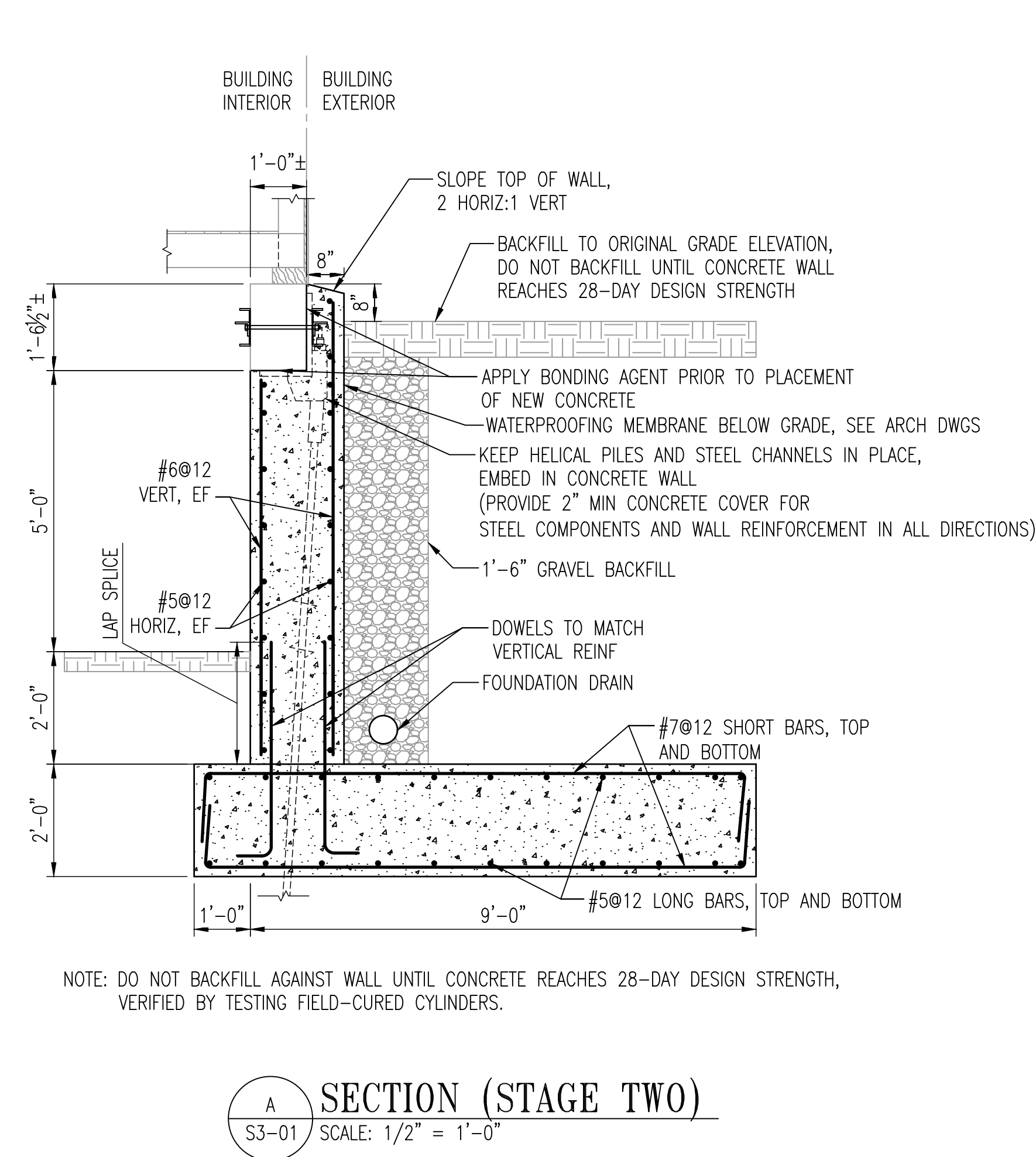
0 2' 4' 8'

SCALE: $1/4" = 1'-0"$

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LAP SPLICE NOTES:

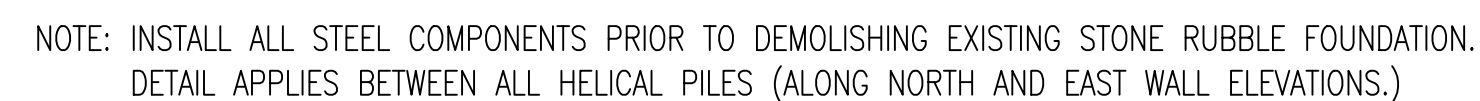
CONCRETE: 5000 PSI COMPRESSIVE STRENGTH (NORMAL WEIGHT CONCRETE), CLASS B LAP SPLICES.

SLAB AND WALL: 6" MINIMUM BAR SPACING WITH CONCRETE COVER = 1.5" CLEAR.

BEAM: MINIMUM CLEAR SPACING BETWEEN BARS = 1.5 DB (BAR DIAMETER). MINIMUM CONCRETE COVER = 1.5" CLEAR. MINIMUM STIRRUP #4@12 PROVIDED.

TOP BAR: HORIZONTAL REINFORCING PLACED SUCH THAT MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE DEVELOPMENT LENGTH OR SPLICE.

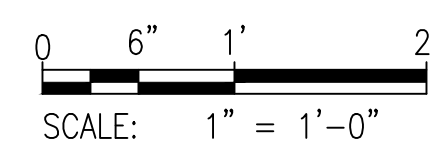
LAP SPLICE LENGTH TABLE




 TYPICAL STEEL SUPPORT OF EXISTING BEAM
 NOT TO SCALE



 **TYPICAL SIGN FOUNDATION DETAIL**
NOT TO SCALE



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